A. Before you begin:

- 1. This Plan Set is intended for use by a general contractor or homeowner without necessarily having to involve a Registered Design Professional.
- 2. Contact your local Building Official, often known as the Building Department, to understand the building permit application process.
- Inquire about:
- a. fees,
- b. how many copies of the plans must be submitted, and
- c. which city inspections are required, see F.3 below.
- 3. The Building Official may also be able to assist with assessing the applicability of this plan set to a home. See Eligibility For Use, Sheet S0.
- 4. Complete the Eliqibility For Use questionnaire on Sheet S0 (Table 1), to determine if this plan set is applicable. A "non-compliant" answer to any question disqualifies the home from using this plan set, unless a Registered Design Professional is involved.

B. Determine your Seismic Design Category (SDC) and Weight Classification:

1. See Sheet S3 and determine the Seismic Design Category (SDC) and Weight Classification for the dwelling. This information will be used to determine which S3.1 sheet is applicable. Note that there are three unique S3.1 sheets for one-story dwellings with differing S_{DS} values and three similar sheets for two-story dwellings. Only one Sheet S3.1 will be applicable to any given dwelling and included within the set of drawings used for submission to the Building Official. Note: You may use S3.1 sheets with S_{DS}= 1.0 for structures located in Puget Sound area.

C. Prepare your plans:

- 1. Draw a scaled plan of the perimeter of the home in the graph layout area provided on Sheet S4, Foundation and Retrofit Layout Plan. Your plan should include the
- a. The location of any obstructions along the perimeter of the foundation that make the retrofitting work difficult or impossible such as fireplaces, water heaters, or utilities. These areas should be avoided when laying out the required retrofitting work.
- b. An arrow to indicate the direction of the span of your floor joists plus the spacing such as "floor joists at 16" on center." This will be helpful when selecting the appropriate details shown on Sheets D1 - D6.
- c. Indicate the height of the tallest cripple wall for each wall line. The minimum required length of retrofitting along each wall line will be based, among other variables, on this height. See the sections in Details 1 and 2 on Sheet D4 for measurement of "cripple wall height."
- d. Dimensions for each length of perimeter wall segment and overall dimensions of wall lines.
- e. An arrow pointing to North.
- f. Label the street side (front) of the home.
- g. See Sheet X1 for an example of a plan sheet submittal.
- h. See Sheets X2 and X3 for additional examples and instructions of how certain items are calculated, such as the length of bracing at offset walls of nonrectangular "T" or "L" shaped dwellings, as noted in Section J, General Notes, Sheet S1.
- i. See Sheets X4 and X5 for illustrations and definitions of terms for retrofit conditions.

D. Gather information to complete the plans:

- 1. Review Sheets S1 and S2 for guidance on materials and installation for the required work.
- 2. Review the Detail Sheets included in this plan set (Sheets D1-D6). Locate the details that most substantially match the home's framing conditions. Not all details or sheets will apply. As a minimum, you should have one detail each for:
- a. The foundation sill to concrete foundation connection (Sheet D1); and
- b. The floor framing to foundation sill connection (Sheet D2); or
- c. Floor framing to cripple wall connection (Sheet D3 and/or Sheet D3.1).
- 3. Differences in existing conditions from those illustrated on the details that result in changes to these drawings will need to be reviewed by a Registered Design Professional. See "Purpose" on Sheet S0 for additional information.
- 4. Once you have chosen the correct (applicable) S3.1 sheet, follow the instructions provided to determine the amount and type of earthquake retrofitting required along each perimeter wall line. Once Steps 1 through 7 of the instructions are completed, document the results within the Retrofit Table as explained in Step 8.
- 5. Refer to Supplemental Technical Notes on Sheet S2 where tie-downs are required.

E. Complete your plans:

- 1. Using the information from the Earthquake Retrofit Schedule on Sheet S3.1, add the following to complete your Foundation and Retrofit Layout Plan on Sheet S4: a. Indicate and dimension the total length of braced wall sections required at each wall line.
- b. Identify the details used for the connections as noted in D.2 above. Indicate the connection type and the minimum number of connectors for each wall line. Conform to Sections L and M of Sheet S1.
- c. Identify the details used for the wood structural panel (Sheets D4 or D5).
- d. If tie-downs are used, identify the details used (Sheet D5).
- e. Identify the detail used for the top plate splice (Sheet D6)
- f. Identify the details used for notching and/or cutouts (Sheet D6.)

F. Submit your plans:

- 1. Submit a permit application and the required number of completed sheets (Sheets S0 through D6) to the Building Official for review. Photographs of the foundation sill, cripple wall, and floor framing conditions may assist the review process.
- 2. Before starting work, the permit holder may be required to schedule a preconstruction inspection with the Building Official to verify that field conditions are consistent with the information provided on the approved plan.
- 3. Inspection(s) by the Building Official may be required for:
- a. Foundation Anchor bolts / Anchor Plate installation,
- Blocking installation.
- c. Wood structural panel on cripple wall, sheathing and nailing.
- d. Metal hardware "connectors" installation.
- e. Tie-downs, and
- f. Final inspection.

Instructions for Use

S0	Cover Sheet
S1	General Notes
S2	Supplemental Technical Notes
S3	Seismic Design Category, Weight Classification, and Connectors
S3.1**	Earthquake Retrofit Schedule - S DS 1.0, One-Story / Two-Story
S4	Foundation and Retrofit Layout Plan
D1	Foundation Sill to Concrete Foundation Connection Details
D2	Floor Framing to Foundation Sill Connection Details
D3	Floor Framing to Cripple Wall Connection Details
D3.1	Floor Framing to Cripple Wall Connection / Foundation Replacement Details
D3.2	Floor Framing to Cripple Wall Connection Details
D4	Wood Structural Panel Installation without Tie-Downs
D5	Wood Structural Panel Installation with Tie-Downs
D6	Vent Openings and Top Plate Details
X1*	Example of Foundation and Retrofit Layout Plan
X2*	Example - Foundation Plan (Dwelling without Tie-Downs)
X3*	Example - Foundation Plan (Dwelling with Tie-Downs)
X4*	Illustration - Cripple Wall Retrofit
X5*	Illustration - Retrofit - No Cripple Wall

^{*} Sheet for reference only. Do not submit to the Building Official.

SHEET LIST

p

Official.

Building

the

submit to

not

ရ

Instructions for Use

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

JUNE 2021



^{**} Only one "S3.1" sheet will be submitted to the Building Official.

PURPOSE

The purpose of this Plan Set is to promote public safety and welfare by reducing earthquake-induced damage to existing wood-frame cripple wall dwellings. The prescriptive designs provided in this Plan Set is being published based on FEMA P-1100, Volume 2. The provisions of this Plan Set address a single vulnerability; see the FEMA P-1100 Prestandard for assessment and extentit methodologies. Like of this Plan Set is intended improve earthquake.

The provisions of this Plan Set address a single vulnerability; see the FEMA P-1100 Prestandard for assessment and retrofit methodologies. Use of this Plan Set is intended improve earthquake performance but is not intended to prevent earthquake damage. For additional information, see https://www.fema.gov/media-library/assets/documents/175158

SCOPE

This Plan Set contains prescriptive provisions for retrofit of wood light-frame crawlspace/basement dwelling anchorage to the foundation and cripple walls. Dwellings shall be considered cripple wall dwellings for purposes of this plan set when:

- The dwelling has unoccupied space or basement below the lowest framed floor.
- The dwelling has cripple walls not exceeding 7'-0" tall (Figure 1).
- The downhill slope does not exceed 30%. (Figures 2 & 3).

ELIGIBILITY

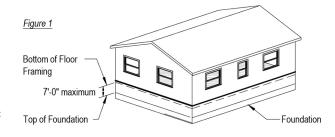
Cripple wall dwellings are permitted to use the prescriptive retrofit provisions of this Plan Set when all questions in Table 1 can be answered with "compliant". For dwellings not eligible to use this Plan Set, see the FEMA P-1100 Prestandard, Section 4.5 for the Simplified Engineered Procedure or consult a Registered Design Professional.

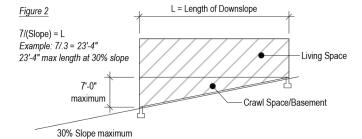
DIFFERING CONDITIONS

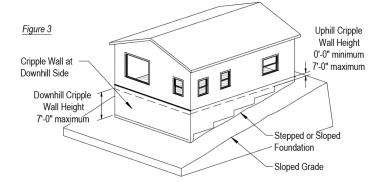
Where a dwelling's actual conditions require modification of the vulnerability-based prescriptive retrofit solutions identified within this plan set, additional or modified details may be generated by a Registered Design Professional and used to supplement the prescriptive procedures of this section. These supplemental details shall be stamped and signed by a Registered Design Professional and approved by the Building Official.

DESIGN BASIS

This Plan Set is based on Chapter 4 of FEMA P-1100 Prestandard. Specific design assumptions are as follows: R = 4.0; $\Omega_a = 1.5$; $S_{DS} = 1.0$, Site Class D.







GENERAL

Cripple Wall Retrofit in accordance with this plan set shall include each of the following for the full extent of the crawlspace perimeter (Figure 4):

- (1) Wood structural panels in accordance with the Earthquake Retrofit Schedule, Sheet S3.1 and details on Sheets D5 & D4 at all nonzero height cripple walls, and
- (2) Foundation sill plate anchorage to the foundation in accordance with the Earthquake Retrofit Schedule, Sheet S3.1 and details on Sheets D1 & D2, and
- 3 Floor framing to cripple wall top plates or floor framing to foundation sill plate connections in accordance with the Earthquake Retrofit Schedule, Sheet S3.1 and details on Sheets D3 & D5.

Any retrofit not incorporating each applicable item at the full crawlspace perimeter shall not be identified as conforming to this Plan Set. All work shall be in accordance with Sheet S1 General Notes.

This Plan Set for strengthening is intended to be approved by the Building Official without requiring additional plans or calculations, except as required for differing conditions.

ASSESSMENT

The retrofit provisions of this Plan Set are intended to apply to dwellings that have been assessed based on the FEMA P-1100 Prestandard methodology with local amendments and found to have a crawlspace dwelling vulnerability.

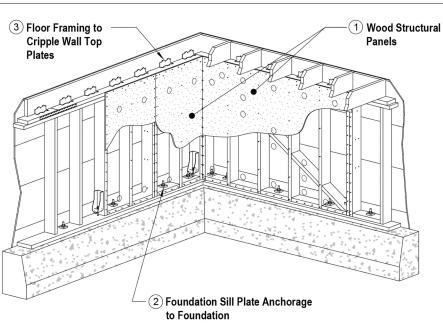


Figure 4

Table 1: ELIGIBILITY FOR USE Non-Compliant To determine if a home qualifies; answer the following: compliant 1. The dwelling is a one- or two-family detached structure or townhouse The dwelling unit is a townhouse and assessment and retrofit will occur for all attached townhouse dwelling units at the same time. 2. The dwelling is a wood light-frame dwelling that is two stories or less (basement OK) 3. The dwelling is a crawlspace/basement and the perimeter (not including porches or other appurtenances) is supported on: a. Cripple walls, or b. Foundation stem walls, or c. Post and pier systems to be retrofitted with cripple walls, or d. Cripple walls or foundation stem walls in combination with a slab on grade foundation. 4. The dwelling has a continuous perimeter foundation (not including porches or other appurtenances), concrete stem walls, or will be retrofitted to have a continuous perimeter foundation. 5. Cripple walls, where they occur, do not exceed 7'-0" in clear height. 6. The maximum slope as measured from the top of foundations along one edge of the home to the other end does not exceed 30% 7. Weight of roofing material shall not exceed 12 psf., except for one-story crawlspace dwellings with clay tile roofing as described in footnote 1 below. 8. Weight of exterior wall finish shall not exceed 10 psf. (Stucco OK), except that masonry wainscots supported on concrete or masonry foundations are permitted to extend up to four feet above the top of foundation. Brick veneer ok for one story condition per note on sheet S3 9. The maximum square footage of the dwelling, excluding areas supported on slabs on grade, do not exceed 3,000 square feet for one story dwellings and 4,000 square feet for two-story dwellings 10. No part of the foundations is constructed of unreinforced masonry or stone. 11. There is no indication that an engineered seismic force-resisting system is present in the dwelling (engineered plans, visible tie-down brackets). If you answered "Compliant" to each of these questions, proceed to Sheet S3.

If you answered "Non-compliant" to any of these questions the home is not eligible to apply this plan set, unless a Registered Design Professional addresses the non-compliant issues in accordance with P-1100 FEMA Prestandard, Section 4.5, Differing Conditions.

Footnote:

1. One story crawlspace dwellings with clay tile that weigh up to 20 psf or full height brick veneer (with light roof material) shall be permitted to be strengthened in accordance with the provisions for two-story heavy construction as noted in the applicable Earthquake Retrofit Tables.

SHEET LIST

- S0 Cover Sheet
 S1 General Notes
- S1 General NotesS2 Supplemental Technical Notes
- S3 Seismic Design Category, Weight Classification, and Connectors
- S3.1 Earthquake Retrofit Schedule
- S4 Foundation and Retrofit Layout Plan
- D1 Foundation Sill to Concrete Foundation Connection Details
- D2 Floor Framing to Foundation Sill Connection Details
- D3 Floor Framing to Cripple Wall Connection Details
 D3.1 Floor Framing to Cripple Wall Connection / Foundation Replacement Details
- D3.2 Floor Framing to Cripple Wall Connection Details
- D4 Wood Structural Panel Installation without Tie-Downs
 D5 Wood Structural Panel Installation with Tie-Downs
- D6 Vent Openings and Top Plate Details

Cover Sheet

Retrofit of Crawlspace Dwellings (Plan Set)

Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

APPLICANT

INFORMATION

LEGAL DESCRIPTION:

JUNE 2021

PARCEL NUMBER:

APPLICANT:

Date: Date: Sheet:

Revision:

eet:

A. CODE

1. All work not otherwise specified shall conform to the locally adopted version of the building code or residential code. Contractor shall comply with all locally adopted building codes and ordinances.

B. GENERAL

- The contractor is responsible for maintaining a safe job site and complying with relevant state and/or federal OSHA standards. Contractor is responsible
 for the means and methods for accomplishing the work shown in this plan set, including any shoring and bracing of existing construction as required to
 safely install new work. Exercise caution working around existing utilities, locate underground utilities before excavating, and arrange for temporary
 disconnection of utilities if necessary.
- 2. All existing under floor ventilation and access shall be maintained.

C. EXISTING CONDITIONS

- 1. Contractor shall confirm that existing conditions match plans and details prior to start of work.
- 2. Contractor shall verify that existing concrete, anchor bolts, wood framing, and other materials that will become part of the work or to which retrofit construction is attached is in reasonably sound condition and free of defects that would substantially reduce the capacity of the material. Where possible, damaged or deteriorated elements shall be repaired in place or supplemented with new elements. Otherwise damaged or deteriorated members shall be replaced. Repair or replacement shall be in accordance with the adopted building or residential code.
- 3. The Owner or Contractor shall verify that the existing concrete within all areas to receive new anchor bolts are in reasonably good condition. Examples of poor concrete quality would include excessive spalling, large rock pockets, cracks extending completely through the footing greater than 1/4" wide (closer than 6'-0" on center on average), or low strength concrete cement or mortar easily scrapable with a metal knife or trowel. Strengthening should be avoided in local areas of poor quality. Where these areas cannot be avoided, or where locations of poor quality.

Table C-1 :	Foundation Verifica	ation Requirements
	Screw Anchor	Adhesive Anchor
Diameter ø	Torque (ft-lbs)	Torque (ft-lbs)
1/2"	35	15
5/8"	50	20

are widespread, the new anchors shall be torque tested in accordance with Table C-1. Where torque tests continue to fail, the existing foundation system shall be replaced locally for a minimum of 30 inches on each side of the proposed anchor location.

D. NOTCHING, BORING AND CUTTING

- Do not cut, bore, or notch structural members except as shown in these drawings or as specifically permitted by the building inspector.
 Exception: Notching and boring of framing shall be permitted as per Chapter 6 of the International Residential Code (IRC)
- 2. When drilling in concrete, do not drill through existing reinforcing steel. If reinforcing steel is hit during drilling, move a minimum of one inch and drill relocated hole. Fill original hole with non-shrink grout.

E. CONCRETE

h

d.

1. Concrete shall have a strength of not less than 3,000 psi at 28 days (design based on 2,500 psi). Concrete mixed on site shall be mixed and placed in accordance with the manufacturer's instructions using potable water.

F. REINFORCING STEEL (REBAR)

- 1. Reinforcing steel shall confirm to ASTM A615 Grade 40 or 60, ASTM A706, or ASTM A996 Type R.
- 2. Reinforcing steel bend radii and other rebar detailing shall be in accordance with Concrete Reinforcing Steel Institute.
- 3. Minimum concrete cover over reinforcing steel:

. Concrete cast against and permanently exposed to soil:	3 inches	
. Formed concrete exposed to weather:	2 inches	
. Concrete not exposed to weather or in contact with soil:	1-1/2 inch	
. Reinforcing steel lap splice lengths:	No. 4	No. 5
 Horizontal bars with more than 12 inches concrete below: 	32 inches	42 inches
Other bars:	24 inches	32 inches

G. STRUCTURAL STEEL

- Structural steel W-sections, plate, bar and miscellaneous steel shall be ASTM A36, A992, or A572. Welding shall comply with AWS D1.1 requirements
 using prequalified welding procedures. All welding shall be conducted by welders certified for the materials and welding procedures used.
- 2. Bolts shall conform to ASTM A-307. Threaded rods shall conform to ASTM A-36.

H. FASTENERS

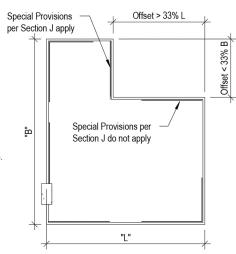
- General
- a. All bolts, nails, and other fasteners in contact with preservative-treated wood or exposed to weather shall be hot-dip galvanized or stainless steel.
- 2. Nails
- a. Unless otherwise noted, all nails specified are to be common nails.
- b. Special care is required when installing nails in existing framing. Where required to avoid splitting of framing, predrill to 75% of nail shank diameter.
- c. Fasteners for wood structural panel sheathing shall be full length 8d common or box nails (0.113" x 2-1/2" minimum size). Drive sheathing nail head flush with face of sheathing.
- d. Do not overdrive, countersink, or otherwise damage the outermost ply when installing nails. A nail is over-driven when it breaks the surface ply. Where nails are overdriven to the point that the veneer is fractured, add one new nail for every (2) overdriven nails. Space new nails between existing.
- 3. Anchor Bolts
- a. Predrill bolt holes to not more than 1/16th inch larger than bolt or anchor bolt to be placed.
- b. At each perimeter wall line, provide a minimum quantity of Foundation Sill Anchors as required by the Earthquake Retrofit Schedule. Place new anchors between 8 and 12 inches from the end of each foundation sill plate and distrubute the remaning anchors as evenly as practical along the wall line.
- c. Provide steel plate washers 0.229 x 3 x 3 inch minimum at all anchor bolts. Centerline of washer should be 1-1/2" to 2" from face of sheathing.
- d. For braced wall sections without tie-downs, provide one of the required anchor bolts within 8" of each end and one additional anchor bolt at each end as noted on Sheet D4.
- e. For braced wall sections with tie-downs, provide one additional anchor bolt within 8" minimum and 12" maximum from tie-down as noted on Sheet D5.

I. WOOD STRUCTURAL PANEL SHEATHING

- 1. Wood structural panels shall be 15/32" plywood sheathing, all veneer, conforming to US voluntary Product Standard PS-1, Exposure I or Exterior Exposure, manufactured with exterior glue, and minimum 4-ply.
- 2. Oriented Strand Board (OSB) shall be 7/16" thick and conform to US Voluntary Product Standard PS 2 with an exposure rating of Exposure 1 or Exterior Exposure, manufactured with exterior glue, and minimum 4-ply.
- 3. Provide 1/8-inch minimum gap at all sheathing panel ends and edges.
- 4. Maintain a minimum edge distance of 3/8" from center of nail to edges of sheathing, studs, or top and sill plates. See Sheet D4 for double stud at sheathing panel joints.
- 5. Braced wall sections closest to the ends of wall lines shall be located as near to the ends as practicable. Braced wall sections may be located away from the ends of a wall line when existing obstructions or limited clearance necessitate such relocations.
- 6. Braced wall sections along the length of a wall line should be nearly equal in length and should be nearly equal in spacing where possible. Using increments of existing stud spacing is expected.
- 7. The length of each braced wall section shall not be less than 24 inches. The length of braced wall sections without tie-downs should be equal to or exceed twice the height of the cripple wall. Exceptions may be permitted when obstructions do not allow braced wall sections of the required length.

J. ADDITIONAL REQUIREMENTS FOR NON-RECTANGULAR DWELLINGS WITH "T OR "L" PLAN CONFIGURATIONS

- 1. Plan configurations other than rectangular such as "T" or "L" shapes that have offsets in the exterior wall lines, within the crawl space plan area greater than 4' and greater than 33% of the largest plan dimension shall meet the following special provisions in that direction:
- a. Foundation sill to foundation connections along offset walls shall have a maximum spacing of 32" on center.
- Floor joist to foundation sill and floor joist framing to the top of cripple wall connections along offset walls shall have a maximum spacing of 16" on center.
- c. Cripple walls, where they occur, shall be sheathed with new wood structural panels. The sheathing shall have a minimum length of 80% of the offset wall length.



K. FRAMING

- 1. Framing shall be Douglas Fir-Larch, HF(Hem Fir), SPF(Spruce Pine Fir) or an approved species having a greater or equal specific gravity.
- 2. Framing in contact with foundations or exposed to weather shall be preservative treated in accordance with AWPA U1 (Commodity Specification A, Use Category 4B). Field treat all cuts, bores and notches per AWPA M-4.

L. CONNECTOR DEVICES

- 1. Connectors shall be pre-engineered pre-manufactured devices, approved by the Building Official and installed in accordance with the manufacturer's instructions.
- Connectors protected from weather shall be provided with a minimum of G90 zinc coating in accordance with ASTM A653. Connectors exposed to weather
 or in contact with preservative treated wood shall be provided with a minimum hot-dip galvanized coating or G185 coating in accordance with ASTM A653,
 and fasteners conforming to ASTM A153.
- 3. Connector devices shall be of the type and size specified in these drawings.
- Connectors required by the Earthquake Retrofit Schedule (Sheet S3.1) shall be distributed equally along the length of each wall line or within the length of the braced wall panel(s).
- $5.\ \mbox{Connector}$ anchors spacing may not be less than $8\mbox{"}$ on center.
- 6. Increase nail or screw length 1/2-inch minimum when installing connectors over wood structural panels

M. POST-INSTALLED ANCHORS

- 1. Post-installed anchors shall be installed in accordance with the manufacturer's installation instructions.
- 2. Adhesive anchors shall be Simpson Strong-Tie SET-XP, HILTI RE 500 SD, CIA GEL 7000C, or approved equivalent.
- 3. Concrete screws shall be Simpson Strong-Tie Titen HD or Powers Fasteners Wedge-Bolt, or approved equivalent.
- 4. See H.3. for additional anchor bolt requirements.

N. PERMITS

1. All work required by this Plan Set shall be permitted through the building department.

O. INSPECTIONS

1. Contractor shall coordinate with the building inspector to ensure that work is accessible for building department inspections, and shall correct non-compliant work as identified by the inspector.

P. SPECIAL INSPECTIONS

- 1. Special inspection by a third party inspector is not required for the following:
- a. Concrete or reinforcing steel for foundations. Design is based on an ultimate concrete strength of 2,500 psi or less.
- b. Installation of cast-in-place or post-installed anchor bolts.
- c. Installation of adhesive anchors for tie-down devices, provided that each anchor is torque-tested in accordance with Table R-2, Sheet S2.
- d. Nailing of wood structural panel shear walls, provided a building department inspection is performed.

PROPERTY Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings **General Notes** Sheet:

Q. PURPOSE OF SUPPLEMENTAL TECHNICAL NOTES

- These Supplemental Techical Notes provide guidance for the installation
 of wood structural panels that use tie-downs and existing foundation
 systems. Tie-downs shall be used where there is insufficient wall length
 to install the length of wood structural panels specified in the Earthquake
 Retrofit Schedule, Sheet S3.1.
- Where "With Tie-down" (as specified on the Earthquake Retrofit Schedule, Sheet S3.1) is used to determine the amount of strengthening required along each wall length, additional visual verification and testing of the existing foundation system may be required to be completed by the owner or general contractor and approved by the Building Official, and documented in Table R-1 prior to commencing any work. Visual verification and testing shall be as noted in Section R.
- 3. Where these requirements are not met, a new foundation system will be required in accordance with detail 6 Sheet D3.1.

R. EXISTING FOUNDATION REQUIREMENTS AND TESTING

- 1. The size of existing foundation systems at the location of new tie-down anchors shall be verified to be at least 15" deep ("D") and 6" wide ("W"). The dimension "D" shall be measured from the bottom of footing to the underside of the existing mudsill. The dimension "W" shall be measured from the top outside face of foundation wall to the inside top face of foundation wall. See Table R-1, item A.1.
- 2. Verification of the overall quality of concrete along any wall line requiring tie-downs shall be made and documented within Table R-1, item B.1. The building Official may require that verification shall be made by use of a minimum of two sacrificial torque tests along each wall line where tie-downs are used. These tests shall consist of installing 1/2" or 5/8" diameter screw-type bolts into the existing concrete and verifying that a value per Table R-2 can be achieved. Torque tests can be performed either by the owner, a general contractor, or a special inspection company or testing agency hired by the owner and as approved by the Building Official.
- Where "Tie-downs" are used to determine the sheathed panel length required along a wall line, each adhesive anchor shall be torque tested in accordance with Table R-2.

S. TIE-DOWN REQUIREMENTS

- Tie-downs shall be Simpson HDU2-SDS2.5, USP Structural Connectors PHD2A, or an equivalent with an allowable tensile load of 3075 lbs or more, installed per manufacturer's instructions
- End studs(s) to which tie-downs are installed, shall be 3x minimum or double 2x. For nailing at double studs, see Sheet D5.
- 3. All tie-downs shall use 5/8"ø (A36) threaded rod adhesive-type anchors with minimum embedment per Detail 1, Sheet D5.

T. MINIMUM INSTALLATION REQUIREMENTS FOR TIE-DOWN ANCHORS TO FOUNDATIONS

- 1. All holes shall be drilled to the specified diameter and depth.
- All holes shall be blown clean of dust with oil-free compressed air for a minimum of 4 seconds.
- 3. All holes shall be cleaned with a nylon brush for a minimum of 4 cycles.
- Blow holes clean of dust with oil-free compressed air for a minimum of 4 seconds. Note: Self vacuuming drill or other approved methods may be used in lieu of above
- Check adhesive cartridge expiration date, open, and install per the manufacturer's instructions.
- 6. Fill the holes 1/2 to 2/3 full, starting at the bottom of the hole to prevent air pockets and withdraw the nozzle as the hole fills up.
- Insert a clean and oil-free anchor turning slowly until the anchor contacts the bottom of the hole.
- Do not disturb the anchor until fully cured. See manufacturer's instructions

Table R-1: Verification of Ex	isting Foun	dation System
Requirement	Yes or N/A	Signature of Owner or Contractor (Owner performing work)
A.1 The size of the existing foundation is greater than or equal to that specified in Section R, item 1.		Signature
B.1 The existing foundation has been verified to be in generally good condition at planned tie-down locations as specified in Section R, item 2. See C(3)/S1 for guideline.		Signature
C.1 When required by the Building Official the capacity of each new tie-down anchor has been verified by passing the torque tests specified in Table R-2.		Signature
D.1 All adhesive anchors were installed per the manufacturer's instructions per the minimum steps as noted in Section T.		Signature

Table R-2:	Foundation Verifica	tion Requirements
	Screw Anchor	Adhesive Anchor
Diameter ø	Torque (ft-lbs)	Torque (ft-lbs)
1/2"	35	15
5/8"	50	20

Supplemental Technical Notes

PROPERTY

(Where Tie-downs are Required)

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

Date:

Seismic Design Category (SDC)

- The first factor taken into consideration when determining the appropriate amount of earthquake strengthening is the anticipated level of seismic shaking or Sps value, which is directly related to the Seismic Design Category (SDC).
- To find the appropriate S_{DS} value, which is either 1.0, 1.2, or 1.5, you must first determine your Seismic Design Category (SDC) by clicking the link below.
 Note: S_{DS} = 1.0 for Puget Sound area.
- a. In your internet browser go to http://www.atcouncil.org/fema-p-1100
- b. Click on one of the five (5) geographic areas listed to find your location on the appropriate map.
- c. Locate your SDC (SDC A-SDC E) by the color contour shown on the map which corresponds to the % g values shown.
- d. For SDC A-D₁, use $S_{DS} = 1.0$.
- e. For SDC D₂, use $S_{DS} = 1.2$ unless the site class can be determined as A, B, or D, in which case use $S_{DS} = 1.0$.
- f. For SDC E, use S $_{DS}$ = 1.5

Note: where your location is on, or close to, the border of two SDC's, it is prudent to choose the higher value.

3. Make a note of the appropriate S _{DS} value. It will be used together with the number of stories the dwelling has to determine the appropriate Earthquake Retrofit Schedule (Sheet S3.1) to use in preparing and submitting the plans. You will only use one of the following two Earthquake Retrofit Schedule Sheets. Do not submit the unused S3.1 sheets to the Building Official.

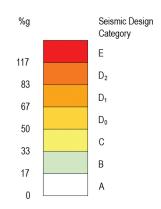


Figure 1. SDC versus %g

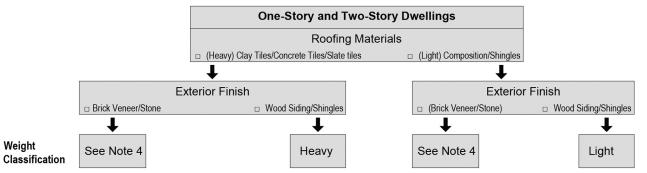
Weight Classification

The next factor used to establish the appropriate amount of earthquake strengthening is the dwelling weight. For the purposed of this Plan Set, two weight classifications (Heavy and Light) have been established as described below. Using the flowchart presented:

- 1. Start with the roofing material then to the the exterior finishes.
- 2. Note the weight classification result for use in the Earthquake Retrofit Schedule, Sheet S3.1.

Specific notes for exterior, interior and roof coverings:

- The "wood siding or shingles" exterior finish category also includes finishes of similar weight, including but not limited to fiber-cement and aluminum siding.
- The "comp or shingles" roofing material category also includes roofing materials of similar weight, including but not limited to roll roofing, built-up felt roofing, single-ply membrane roofing, and metal roofing.
- 3. The exterior finish, roofing material, and interior finish categories are intended to be identified based on the predominant materials used in construction. Where interior or exterior finishes vary, a heavier type finish shall be assumed where 25% or more of the heavier finish type exists within the dwelling.
- 4. Structures with both brick veneer/stone veneer and heavy roof assembly should consult with a registered design professional and are outside the scope of this document.
- Structures with both brick veneer/stone veneer and light roof assembly are limited to one story above concrete foundation and shall proceed using the two-story tables for determining wall panel length and anchorage/fastener spacing.



Connectors

- 1. Manufacturer's model numbers and installation instructions are subject to change. Verify and follow manufacturer's written instructions.
- 2. Connector images are general in nature only. Individual manufacturer's connectors may vary.
- 3. Any of the connectors listed within a particular group may be used for strengthening the particular condition.
- 4. This Plan Set was developed using the lowest listed manufacturer's capacity within a particular group.
- 5. Where connectors listed within the applicable Earthquake Retrofit Schedule will not fit within a particular wall line due to limitations in length, alternate connections may be substituted but shall be designed or selected by a registered design professional and approved by the Building Official.

<u></u>	TYPE B Simpson Stron	•	FRFP W/ (5)SDS		181			
	USP Structura TYPE C/NA; R Simpson Stror USP Structura	Reserved lg-Tie	FRFP SFA8		157 106 875	5#	RESS:	
\$	TYPE D Simpson Stron Simpson Stron USP Structura	ıg-Tie	L70 A35 AC7	- 1	740 650 725	#	PROPERTY ADDRESS	
	TYPE E Simpson Stron USP Structura		L90 AC9		925 905	-		
	TYPE F (Note: Simpson Stron USP Structural	g-Tie	H10A RT16A		565 800		ification	IIICauoii,
	TYPE G Simpson Stron USP Structural	•	LTP4 MP4F		625 660		itcoifiaacl Otto	III Ciass
	TYPE H Simpson Stron USP Structura		L30 A3		250 590		200///	y, יי שיי nnectors
	TYPE S1 Simpson Strong USP Structural		MSTA36/ (2)CS16 MSTA36		170: 206		20040	and Cor
	ANCI	HOR BC	LTS	,			5.	- 6
MANUFA	CTURER	MODEI	-	I	DEF		2	ָ ק
SCREW-TYPE Simpson Stron Powers Faster	ıg-Tie	Titen HD Wedge-Bolt		3-1/2 3-3/8	2"	3-1/2" 3-3/8"	Coiemic	Seisille Design Category
ADHESIVE Simpson Stron Hilti USP Structura	•	Threaded Rod SET XP, ATXI HIT-HY 200 CIA GEL 7000	P Adhesive	4-1/4 2-3/4 2-3/4	4"	5" 3-1/8" 3-1/8"		
Quor	TIE blemental Tech	E-DOWN		Section	n 9			
IMAGE	MANUFA		MODEL		CAF	PACITY ASD)		
_		_					i	/

CONNECTORS

MODEL

URFP

SRC

CAPACITY

1530#

1450#

MANUFACTURER

USP Structural Connectors

Simpson Strong-Tie

TYPE A

IMAGE

Note: Tie-down capacities listed above are ASD and based on manufacturer's data. The allowable ASD capacity used for development of this plan set has been reduced to 3000# based on anchorage to existing foundation systems. Tie down anchors must be installed per Detail 1, Sheet D5.

Simpson Strong-Tie

USP Structural Connectors

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

Date:

3075#

3215#

HDU2

PHD2A

CI.

S3

				E	ARTHQ	UAKE R	ETROF	IT SCH	EDULE	(S _{DS} = 1	.0 Seism	ic) ON	IE-ST	ORY			
1	2	3	(5)			h Each of		ed Wall S	ections R			Numb	er of Fo	oundat ch Peri	meter Wa		Anchors
Weight Category		that applies					ood Struc	ctural Pan	els				ndatio			to Cripple or	
ght C		LOW	4	I	· ·	ple Wall I						/	Anchor	s	Floor to	Founda	tion Sili
Wei	Total Area in Square Feet	⊠ Mark	up to 1' Without Tie- downs	1'-1" to 2' Without Tie- downs	2'-1" t Without Tie- downs	o 4'-0" With Tie- downs	4'-1" to Without Tie- downs	o 6'-0" With Tie- downs	6'-1" to Without Tie- downs	o 7'-0" With Tie- downs	Panel Edge Nailing	Type "A/B"	1/2"ø Bolt	5/8"ø Bolt	Type "D"	Type "E" or "F"	Type "G"
	up to 800		5.3'	5.3'	8.0'	5.3'	9.3'	5.3'	9.3'	6.7'	4"	4	7	5	11	10	14
_	801 to 1000		6.7'	6.7'	8.0'	6.7'	10.7'	6.7'	10.7'	8.0'	4"	5	8	6	13	12	16
, uctio	1001 to 1200		6.7'	6.7'	9.3'	6.7'	10.7'	8.0'	12.0'	8.0'	4"	6	10	7	15	14	19
Story	1201 to 1500		8.0'	8.0'	10.7'	8.0'	13.3'	9.3'	13.3'	9.3'	4"	7	12	8	18	17	22
1-Story Light Construction	1501 to 2000		9.3'	10.7'	13.3'	10.7'	14.7'	10.7'	16.0'	12.0'	4"	9	15	10	23	22	29
ij	2001 to 2500		12.0'	12.0'	14.7'	12.0'	17.3'	12.0'	18.7'	13.3'	4"	10	18	12	27	26	35
	2501 to 3000		14.7'	14.7'	16.0'	14.7'	18.7'	14.7'	20.0'	16.0'	4"	12	21	14	32	31	40
	up to 800		5.3'	6.7'	8.0'	5.3'	10.7'	6.7'	10.7'	8.0'	2"	6	10	7	15	14	18
LC.	801 to 1000		6.7'	8.0'	9.3'	6.7'	12.0'	8.0'	12.0'	9.3'	2"	7	11	8	17	17	22
, ructic	1001 to 1200		6.7'	8.0'	10.7'	8.0'	12.0'	9.3'	13.3'	10.7'	2"	8	13	9	20	19	25
Story	1201 to 1500		8.0'	9.3'	12.0'	9.3'	14.7'	10.7'	14.7'	12.0'	2"	9	15	11	24	23	30
1-Story Heavy Construction	1501 to 2000		9.3'	10.7'	14.7'	10.7'	16.0'	12.0'	17.3'	13.3'	2"	11	19	13	30	29	38
H H	2001 to 2500		10.7'	13.3'	16.0'	12.0'	18.7'	14.7'	20.0'	16.0'	2"	13	23	16	36	34	45
	2501 to 3000		12.0'	14.7'	17.3'	13.3'	20.0'	16.0'	21.3'	17.3'	2"	16	27	18	41	40	53

Notes:

- 1. Anchor bolts and Connectors shown in the Earthquake Retrofit Schedule are the minimum required per wall line, placed within the length of strengthening where possible and spaced as equally along each wall line as possible
- 2. Tie-downs: If your foundation meets the criteria, you may choose the tie-down option to decrease the required length of strengthening. This may be required where the length of the wall without tie-downs specified in this schedule is longer than can be accommodated by existing conditions. However, there is a level of uncertainty when dealing with existing foundations, therefore, where possible, longer lengths of strengthening, without tie-downs, are preferred. (See Supplemental Technical Notes, Sheet S2 to verify the existing foundation is suitable and meets criteria.)
- 2(a). Where the available wall length with tie-down per wall line is less than table required lengths; It is acceptable to provide braced walls with tie-down for 80% of the wall line length. The required pony wall connections shall be minimum of 1/2" bolts at 18" o.c. (or equivalent) and fasteners at 12" o.c.
- 3. Connector Type "F" should be used as an alternative only if joists have blocking on both sides and where accessibility makes the use of Types "D" or "E" impractical.
- 4. Any of the connectors listed within a particular group and as shown on Sheet S3 may be used for strengthening the particular condition.
- 5. This Plan Set was developed using the lowest listed manufacturer's capacity within a particular group. Required number of connectors on the Earthquake Retrofit Schedule may be found to have an acceptable spacing where an alternate connector is used. Any such substitution can only be designed or specified by a Registered Design Professional.
- 6. Foundation sill anchor types A, B, and C should not be used with cripple walls over 2 feet.

INSTRUCTIONS

- (1) Locate the section that matches your home's construction. Use the flowchart on Sheet S3 to determine "Weight Category".
- (2) Find the home's Total Floor Area in the schedule, this number should be at least as large as the number listed below. Do not use a smaller number, even if it is closer.
 - a. Approximate 1st floor area over crawlspace / basement:
 (Do not include areas built over slab-on-grade.)
 Note: Basement/ crawl space not included in total area.
- (3) Check the box that matches your home's construction type, number of stories, and total floor area. You will use information in this row of the schedule to determine length of wood structural panels, nailing requirements, quantities of hardware, etc.
- (4) Measure the maximum height of the cripple wall along each wall line of the house.
- (5) Determine the length of wood structural panel bracing required. The columns contain the length of required bracing, depending on the height of the cripple wall. The length of bracing is given for cripple wall heights of zero to 1', 1' to 2', 2' to 4', 4' to 6', and 6' to 7'. Furthermore, choices are given for bracing without tie-downs and with tie-downs. If the cripple wall height changes along the length of the wall, use the average height to determine the required bracing length. Follow the row across from the total floor area that you checked for your home (in Step 2) to find the bracing length for the cripple wall height on each side of the house. Review General Notes, Sheet S1, Section J for instructions at non-rectangular "T" or "L" shaped dwellings. Check boxes where Special Provisions apply.
- (6) Determine the number of Foundation Sill Anchors required. The columns show the number of anchors required, depending on whether you use Types A through C, or 1/2"ø or 5/8"ø anchor bolts. (ø = diameter of the bolts.) See Sheet S3.
- (7) Determine the number of Floor to Cripple Wall or Foundation Sill connectors. The columns indicate how many framing connectors are required, depending on whether you use Types D, E, F, or G. See Sheet S3.
- (8) Complete the Retrofit Summary for your project. Fill in the lengths found in (5). Check the boxes for the anchor and connector types you plan to use. The length of new cripple wall sheathing should be distributed along a wall line either in one full length or in a maximum of two panel lengths of approximately equal length (offset walls can have three). If you intend to use tie-downs, check the box for tie-downs for each wall line where use is intended. Check the box on line 4, and read the Supplemental Technical Notes for additional information. Where the length of required panel does not fit within the available length, the dwelling must have an engineered solution. Alternately, if 80% of the length along any particular wall can be sheathed, then that wall line shall be considered acceptable and an engineered solution is not required. (See note 2(a))

Earthquake Retrofit Schedule S _{DS}= 1.0 One-Story

Dwellings (Plan Set) ssessment and Retrofit of mily Dwellings

f Crawlspace Dw. y-Based Seismic Assess One- and Two-Family L

Retrofit of (

RETROFIT SUMMARY BRACING, ANCHORS, CONNECTORS, AND TIE-DOWNS

Minimum re	quired le	ngth of Wo	ood Structural Pa	inels	per wall line:	(check boxes w	here condition applies)
North Wall		_ft x2 □	with tie-downs		with Special	l Provisions	
East Wall		_ft x2 □	with tie-downs		with Special	l Provisions	
South Wall		_ft x2 □	with tie-downs		with Special	l Provisions	
West Wall		_ft x2 □	with tie-downs		with Special	l Provisions	
2. Panel Edge	Nailing	" (on center.				
3. New Founda	ation Sill	Anchorag	e:			Additional type:	s/quantity (if used):
North Wall	Type: _		Min required:			Туре:	_ Min required:
East Wall	Type: _		Min required:			Туре:	_ Min required:
South Wall	Type: _		Min required:			Туре:	_ Min required:
West Wall	Type: _		Min required:			Туре:	_ Min required:
4. Floor Framir	ng Conne	ectors (to	Cripple Wall or Fo	ounda	ntion Sill):	Additional type:	s/quantity (if used):
North Wall	Type: _		Min required:			Туре:	_ Min required:
East Wall	Type: _		Min required:			Туре:	_ Min required:
South Wall	Type: _		Min required:			Туре:	_ Min required:
West Wall	Type:		Min required:			Type:	Min required:

5.
Check this box if tie-downs and SUPPLEMENTAL TECHNICAL NOTES on Sheet S2 will be used.

Date:

S3.1

				EA	ARTHQ	JAKE R	ETROF	IT SCHI	EDULE	(S _{DS} = 1.	0 Seismi	c) TW	O-ST	ORY			
1	2	3 se	(5)		Lengt		Two Brac Each Per		Sections Ro	equired		A	Anchor	s at Ea	ndation (ch Perim ributed Al	eter Wall	Line
Weight Category		that applies				W	ood Struc	ctural Pan	els			6 Fou	ndatio	n Sill		to Crippl or	
r C		NO.	4		Crip	ple Wall I	Height					1	Anchor	S	Floor to	Founda	tion Sill
/eig		Mark		1'-1" to 2'		o 4'-0"	4'-1" to		6'-1" t	o 7'-0"						Туре	
S	Total Area in Square Feet	×	Without Tie- downs	Without Tie- downs	Without Tie- downs	With Tie- downs	Without Tie- downs	With Tie- downs	Without Tie- downs	With Tie- downs	Panel Edge Nailing	Type "A/B"	1/2"ø Bolt	5/8"ø Bolt	Type "D"	"É" or "F"	Type "G"
드	up to 1600		8.0'	8.0'	10.7'	8.0'	12.0'	9.3'	13.3'	9.3'	4"	7	11	8	17	17	22
y	1601 to 2000		9.3'	9.3'	12.0'	9.3'	13.3'	10.7'	14.7'	10.7'	4"	8	13	9	20	19	26
2-Story : Construction	2001 to 2400		10.7'	10.7'	13.3'	10.7'	14.7'	10.7'	16.0'	12.0'	4"	9	15	10	23	22	29
2 Light C	2401 to 3000		12.0'	12.0'	14.7'	12.0'	17.3'	13.3'	18.7'	13.3'	4"	10	18	12	27	26	34
تز	3001 to 4000		14.7'	14.7'	17.3'	16.0'	20.0'	16.0'	21.3'	16.0'	4"	13	22	15	34	32	43
uo	up to 1600		9.3'	9.3'	12.0'	9.3'	13.3'	10.7'	14.7'	12.0'	2"	9	16	11	24	23	30
y tructi	1601 to 2000		9.3'	10.7'	13.3'	10.7'	14.7'	12.0'	16.0'	13.3'	2"	11	18	13	28	27	35
2-Story / Construction	2001 to 2400		10.7'	12.0'	14.7'	10.7'	16.0'	13.3'	17.3'	14.7'	2"	12	21	14	32	31	41
2. Heavy (2401 to 3000		12.0'	13.3'	16.0'	13.3'	18.7'	14.7'	18.7'	16.0'	2"	14	25	17	38	37	48
유	3001 to 4000		13.3'	16.0'	18.7'	14.7'	21.3'	17.3'	22.7'	18.7'	2"	18	31	21	48	46	60

Notes:

- 1. Anchor bolts and Connectors shown in the Earthquake Retrofit Schedule are the minimum required per wall line, placed within the length of strengthening where possible and spaced as equally along each wall line as possible. Note that where using 1/2" or 5/8" bolts, one additional anchor is required at the end of each braced wall panel per Sheet D4.
- 2. Tie-downs: If your foundation meets the criteria, you may choose the tie-down option to decrease the required length of strengthening. This may be required where the length of the wall without tie-downs specified in this schedule is longer than can be accommodated by existing conditions. However, there is a level of uncertainty when dealing with existing foundations, therefore, where possible, longer lengths of strengthening, without tie-downs, are preferred. (See Supplemental Technical Notes, Sheet S2 to verify the existing foundation is suitable and meets criteria.)
- 2(a). Where the available wall length with tie-down per wall line is less than table required lengths; It is acceptable to provide braced walls with tie-down for 80% of the wall line length. The required pony wall connections shall be minimum of 1/2" bolts at 18" o.c. (or equivalent) and fasteners at 12" o.c.
- 3. Connector Type "F" should be used as an alternative only if joists have blocking on both sides and where accessibility makes the use of Types "D" or "E" impractical.
- 4. Any of the connectors listed within a particular group and as shown on Sheet S3 may be used for strengthening the particular condition.
- 5. This Plan Set was developed using the lowest listed manufacturer's capacity within a particular group. Required number of connectors on the Earthquake Retrofit Schedule may be found to have an acceptable spacing where an alternate connector is used. Any such substitution can only be designed or specified by a Registered Design Professional.
- 6. Foundation sill anchor types A, B, and C should not be used with cripple walls over 2 feet.

INSTRUCTIONS

- (1) Locate the section that matches your home's construction. Use the chart on Sheet S3 to determine "Weight Category".
- (2) Find the home's Total Floor Area in the schedule, this number should be at least as large as the number calculated in 2.c. Do not use a smaller number, even if it is closer.
 - a. Approximate 1st floor area over crawl space / basement: (Do not include areas built over slab-on-grade.)
 - Approximate 2nd floor area over crawl space / basement:
 (Do not include areas where the 1st floors over slab-on-grade.)
 - c. Total approximate square footage:

Note: Basement/ crawl space not included in total area.

(line a + line b)

- (3) Check the box that matches your home's construction type, number of stories, and total floor area. You will use information in this row of the schedule to determine length of wood structural panels, nailing requirements, quantities of hardware, etc.
- (4) Measure the maximum height of the cripple wall along each wall line of the house.
- (5) Determine the length of wood structural panel bracing required. The columns contain the length of required bracing, depending on the height of the cripple wall. The length of bracing is given for cripple wall heights of zero to 1', 1' to 2', 2' to 4', 4' to 6', and 6' to 7'. Furthermore, choices are given for bracing without tie-downs and with tie-downs. If the cripple wall height changes along the length of the wall, use the average height to determine the required bracing length. Follow the row across from the total floor area that you checked for your home (in Step 3) to find the bracing length for the cripple wall height on each side of the house. Review General Notes, Sheet S1, Section J for instructions at non-rectangular "T" or "L" shaped dwellings. Check boxes where Special Provisions apply.
- (6) Determine the number of Foundation Sill Anchors required. The columns show the number of anchors required, depending on whether you use Types A through C, or 1/2"ø or 5/8"ø anchor bolts. (ø = diameter of the bolts.) See Sheet S3.
- 7 Determine the number of Floor to Cripple Wall or Foundation Sill connectors. The columns indicate how many framing connectors are required, depending on whether you use Types D, E, F, or G. See Sheet S3.
- (8) Complete the Retrofit Summary for your project. Fill in the lengths of Wood Structural Panels found in 5. Fill in the type and quantity of anchors used on each wall line. The length of new cripple wall sheathing should be distributed along a wall line either in one full length or in a maximum of two panel lengths of approximately equal length (offset walls can have three). If you intend to use tie-downs, check the box for tie-downs for each wall line where use is intended. Check the box on line 5, and read the Supplemental Technical Notes for additional information. Where the length of required panel does not fit within the available length, the dwelling must have an engineered solution. Alternately, if 80% of the length along any particular wall can be sheathed, then that wall line shall be considered acceptable and an engineered solution is not required. (See note 2(a))

RETROFIT SUMMARY BRACING, ANCHORS, CONNECTORS, AND TIE-DOWNS 1. Minimum required length of Wood Structural Panels per wall line: (check boxes where condition applies) North Wall ft x2 u with tie-downs u with Special Provisions East Wall ft x2 u with tie-downs with Special Provisions with Special Provisions South Wall ft x2 with tie-downs West Wall ft x2 u with tie-downs u with Special Provisions 2. Panel Edge Nailing on center. 3. New Foundation Sill Anchorage: Additional types/quantity (if used): Min required: North Wall Min required: Min required: East Wall Min required: South Wall Min required: Type: Min required: Type: Min required: West Wall Type: Min required: Type: 4. Floor Framing Connectors (to Cripple Wall or Foundation Sill): Additional types/quantity (if used): North Wall Min required: Min required East Wall Min required: Min required: Type: Type: South Wall Min required: Min required: Type: Type: West Wall Min required: Min required: Type: Type:

5.

Check this box if tie-downs and SUPPLEMENTAL TECHNICAL NOTES on Sheet S2 will be used.

Earthquake Retrofit Schedule $S_{DS} = 1.0$ Two-Story

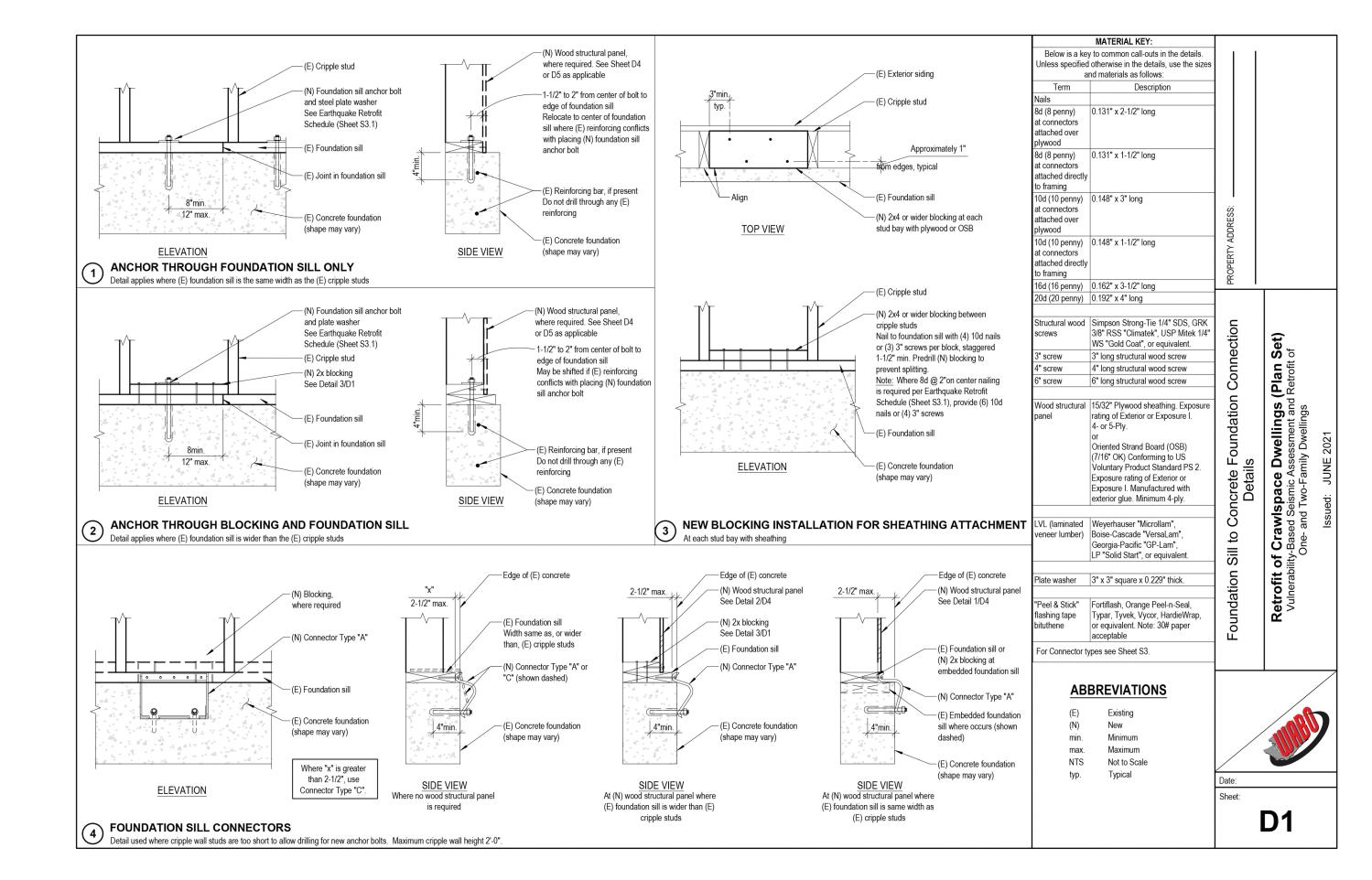
Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

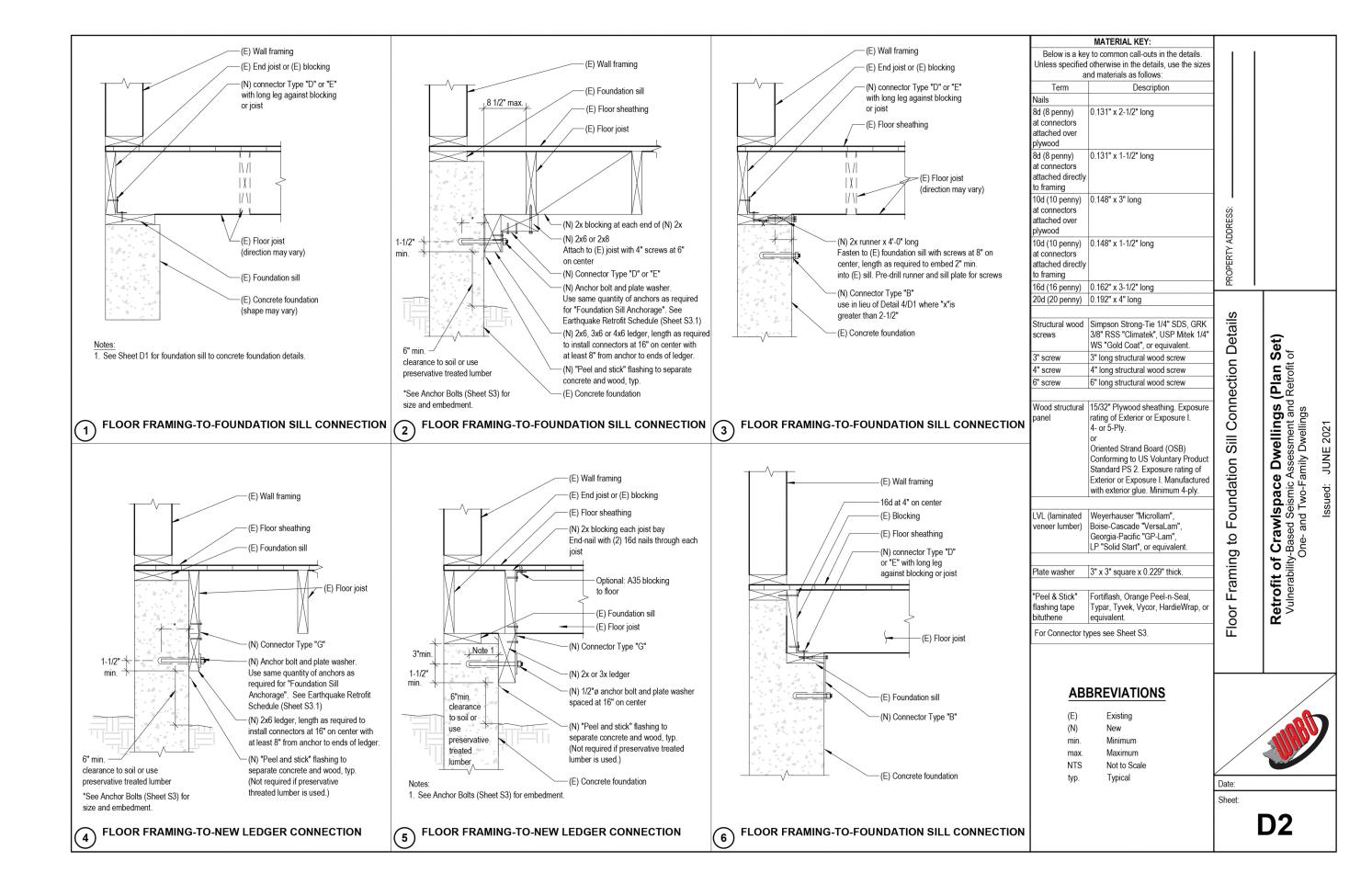
Date:

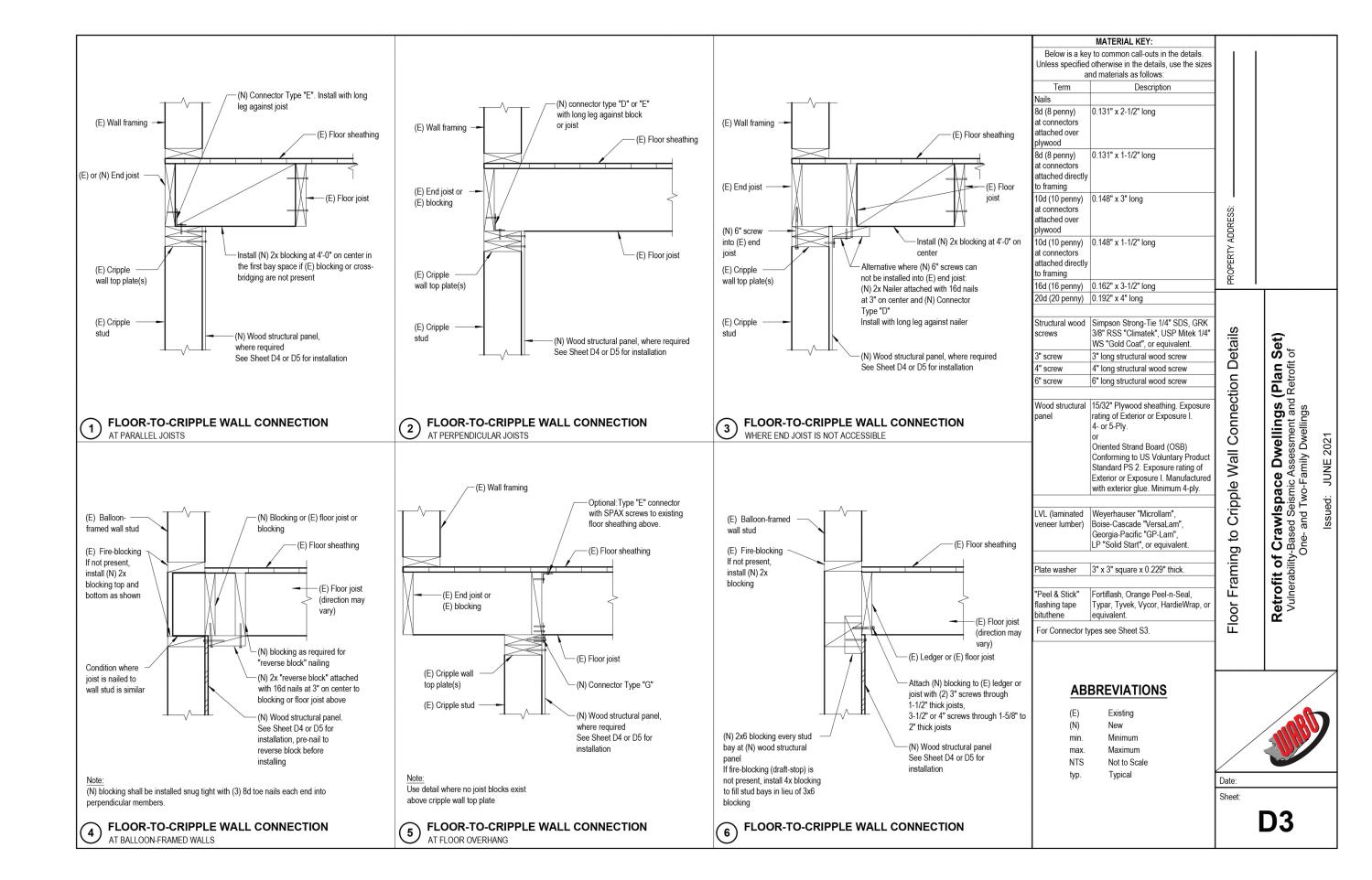
Date: Sheet:

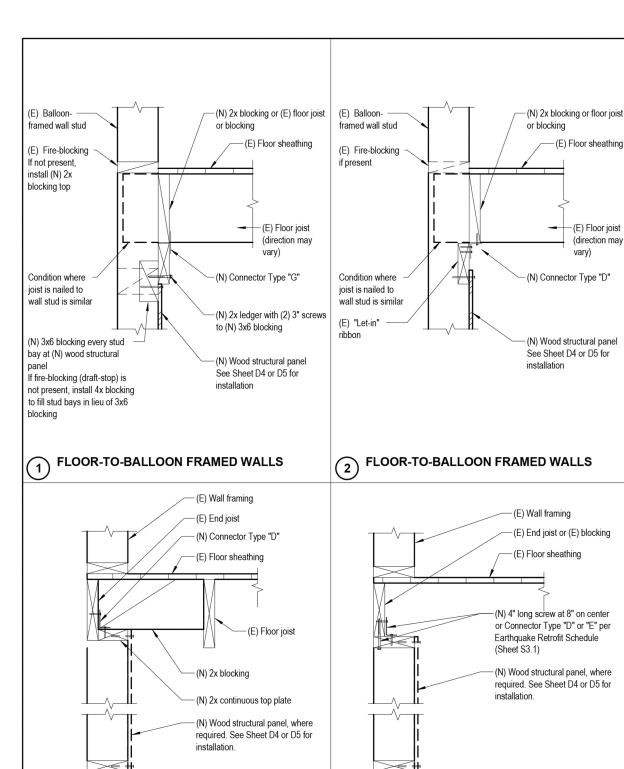
S3.1

	Notes: 1. See Retrofit Sum																			: : :		, <u></u>								: : :
,	nmary on Shee		: :																	: :										1 : :
	t S3.1 for m	 	:	 	:				:				:			:				:				<u> </u>	:	:				:
	inimum retro		:		:	:			:									:		:	:				:		:			:
0 1	fitting requi					:	· · · · · · · · · · · · · · · · · · ·		: :								:			: :			:				:			
	rements.	.	:						- :	.			1							:										:
			: :						: :		<u> </u>	<u>.</u>	: :							: :			<u>.</u>	: :		: :	: :			: :
			1 :						:		[<u>.</u>		1			:	1			:										
			: :		: :				: :							: :				: :						: :				: :
					: :				: :				: :							: :										: :
			:				: .						:			:		.	.	:						:	. : .	.	.	:
			: :						: :				: :			: :				: :				<u> </u>		: :	• • • • • • • • • • • • • • • • • • • •			
			- :		:				:				:			:				:						:				:
ON			:		:		:		:							:				:	:				:	:				:
NE SQUARE =			: :		: :				: :											: :						: :				: :
:FEET																				: : :				: : :						: : :
											. [: : :		. [.	.	1 : : :
		.	:			.			- :				:							:						:				1 :
			: :		: :				: :				: :			: :				: :						: :				
North Arrov			: :		: :															1 1					: :					. :
)	\						: :	<u></u>	: :											: :	:				: :		: :			: :
	Revision:								: :									. :		: :						: :	. :			: :
	<u>Date:</u>		<u> </u>		: : :				<u> </u>											: : :							• • • • • • • • • • • • • • • • • • • •			: : :
SHEEL.	Date:					P _O	_ 	atic	ī a	nd F			La	/out	Foundation and Retrofit Layout Plan	<u>_</u>				PROP	H	, ADDRESS:	ESS:							
S4		cho-			A .	vitrol Vulne	fit c erabi	PE G	rawlsp sased Sei ne- and T Issued:	Isp I Seis nd Tv	ace smic vo-F	ace Dwelsmic Assess vo-Family D	velli sssm y Dwr	ings ent a elling	Retrofit of Crawlspace Dwellings (Plan Set) Vulnerability-Based Seismic Assessment and Retrofit of One- and Two-Family Dwellings Issued: JUNE 2021	lan tetrof	Set iit of	=												





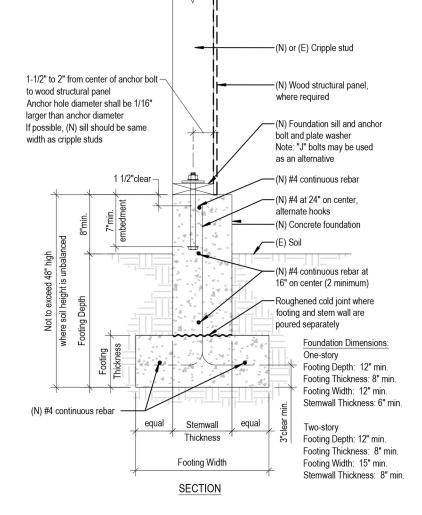




(E) Concrete foundation

See Sheet D3.2 for alternate.

CRIPPLE WALL WITHOUT TOP PLATE



(E) Concrete foundation

CRIPPLE WALL WITH SINGLE TOP PLATE

- 1. Contact Building Official to verify applicability
- 2. Where frost conditions occur, the minimum depth shall extend below the frost line.
- 3. Footing to be deepened as required to bear on firm soils.
- 4. When expansive soil is known to exist, the foundation depth and reinforcement shall be as approved by the Building Official.
- 5. The ground surface along the interior side of the foundation is permitted to be excavated to the elevation of the top of the footing.
- 6. Where (N) foundations are placed adjacent to (E) foundations, connect (N) and (E) foundations with (3) #4 x 3'-6" Embed dowels 8" minimum into the (E) foundation with adhesive. See General Notes, Section M.
- 7. A geotechnical report or modified foundation may be required at locations with expansive or liquefiable soils or sites with potential for sliding.

ABBREVIATIONS

MATERIAL KEY: Below is a key to common call-outs in the details. Unless specified otherwise in the details, use the size and materials as follows:

0.131" x 2-1/2" long

0.131" x 1-1/2" long

0.148" x 3" long

Structural wood | Simpson Strong-Tie 1/4" SDS, GRK

Wood structural 15/32" Plywood sheathing. Exposure rating of Exterior or Exposure I.

4- or 5-Plv.

LVL (laminated | Weverhauser "Microllam",

veneer lumber) | Boise-Cascade "VersaLam",

equivalent.

3/8" RSS "Climatek", USP Mitek 1/4"

WS "Gold Coat", or equivalent.

3" long structural wood screw

4" long structural wood screw

6" long structural wood screw

Oriented Strand Board (OSB)

Conforming to US Voluntary Product

Exterior or Exposure I. Manufactured

Standard PS 2. Exposure rating of

with exterior glue. Minimum 4-ply.

Georgia-Pacific "GP-Lam",

3" x 3" square x 0.229" thick.

Fortiflash, Orange Peel-n-Seal

Typar, Tyvek, Vycor, HardieWrap, or

LP "Solid Start", or equivalent.

10d (10 penny) 0.148" x 1-1/2" long

16d (16 penny) 0.162" x 3-1/2" long

20d (20 penny) 0.192" x 4" long

Term Nails

8d (8 penny)

at connectors

attached over

8d (8 penny)

at connectors

at connectors

attached over plywood

at connectors

to framing

3" screw

" screw

screw "

Plate washer

'Peel & Stick"

lashing tape

bituthene

attached directly

attached directly to framing 10d (10 penny)

plywood

Description

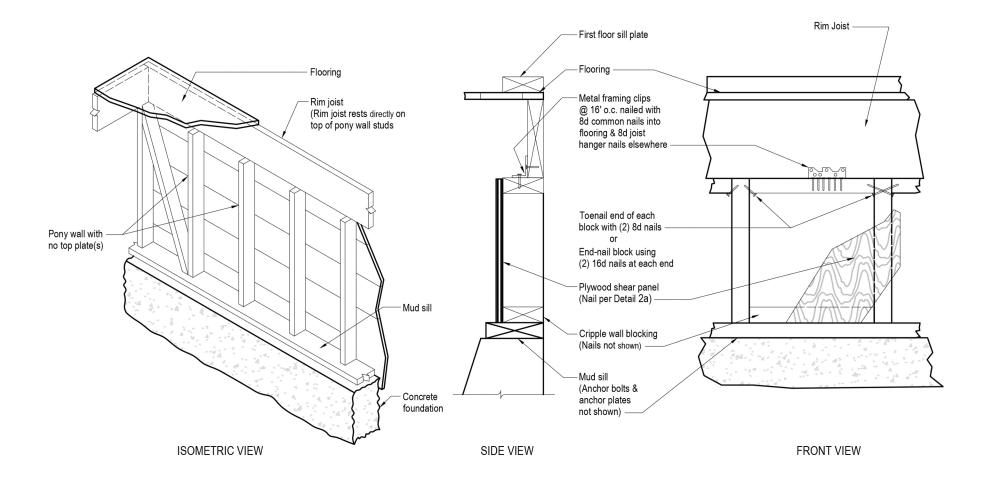
(E) Existing (N) New Minimum min. Maximum max NTS Not to Scale typ. Typical

Crawlspace Dwellings (Plan Set)
Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings Cripple Wall 9 One-Framing Retrofit of (Floor

Connection Details

D3.1

CONCRETE FOUNDATION FOR SECTION REPLACEMENT



PROBLEM: No pony wall top plate(s)

SOLUTION:

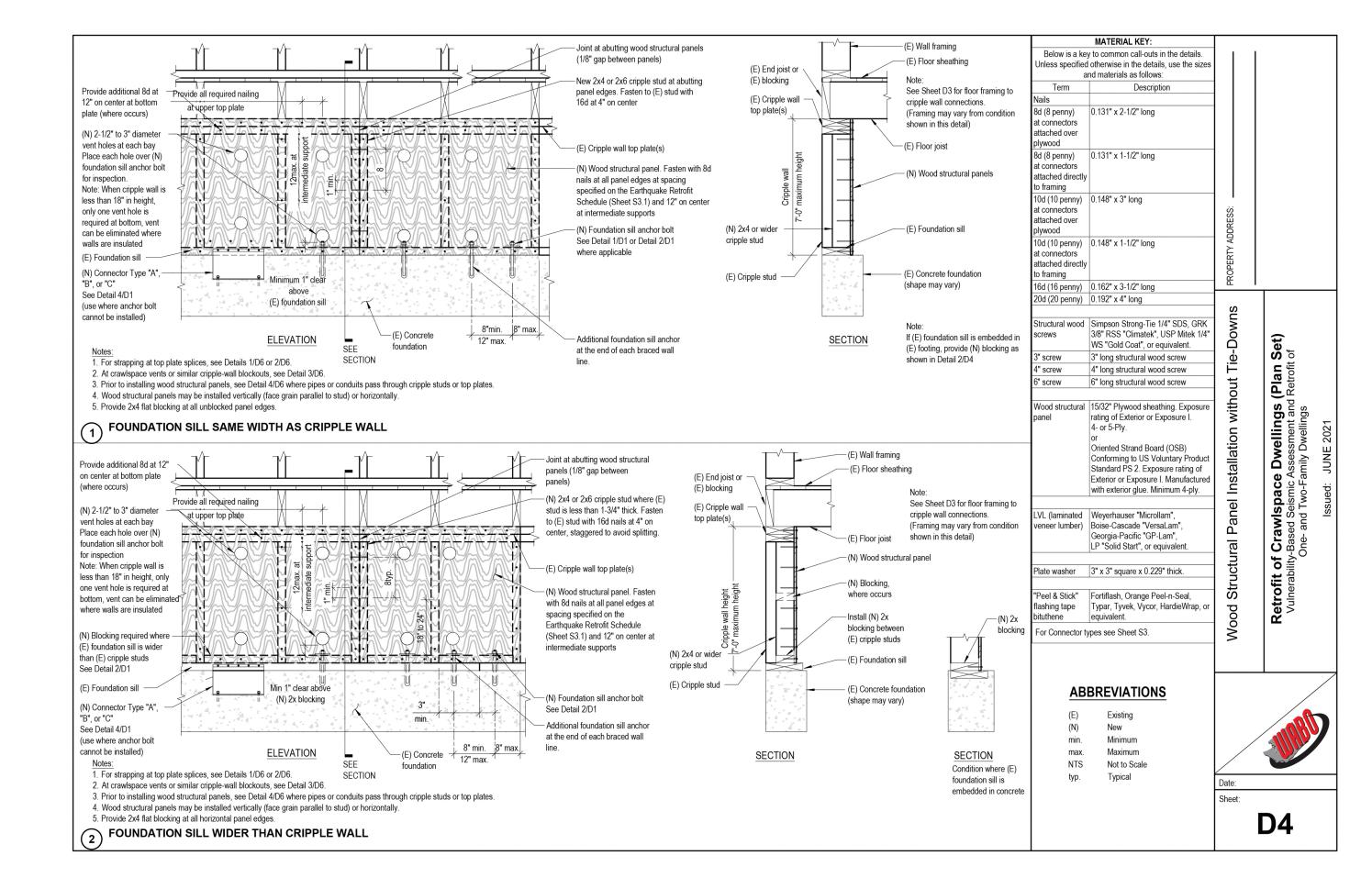
Framing modifications are necessary to provide the required nailing surfaces for the plywood shear panels and to ensure connections which complete the load path between the pony wall and the floor system.

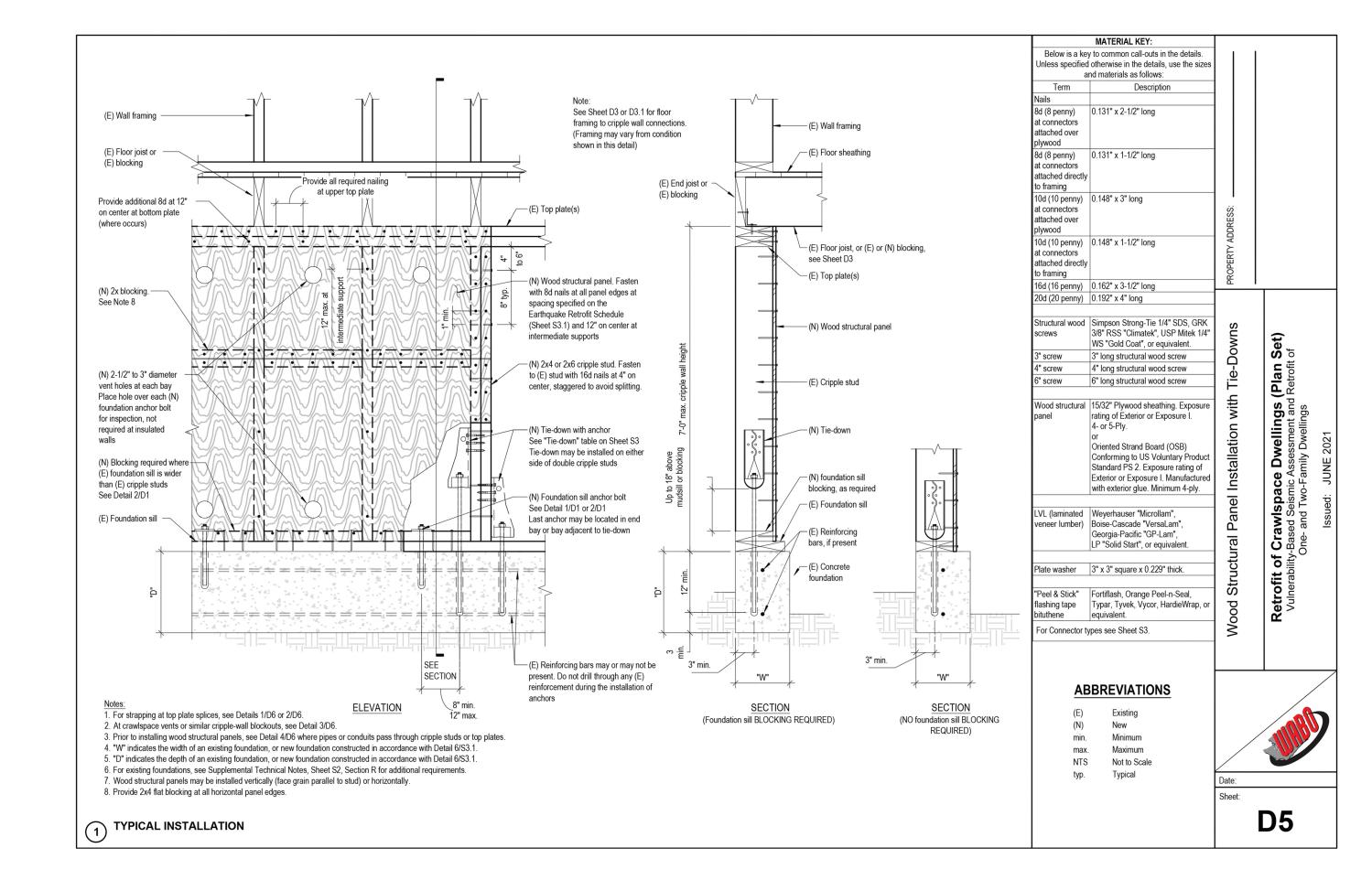
	MATERIAL KEY:			
	y to common call-outs in the details.	1 1		
	otherwise in the details, use the sizes			
	nd materials as follows:			
Term	Description			
Nails				
8d (8 penny)	0.131" x 2-1/2" long			
at connectors attached over				
plywood				
8d (8 penny)	0.131" x 1-1/2" long			
at connectors	0.131 X 1-1/2 long			
attached directly				
to framing				
10d (10 penny)	0.148" x 3" long	'		
at connectors		.SS:		
attached over		REG		
plywood				
10d (10 penny)	0.148" x 1-1/2" long	_ ×		
at connectors		I III I		
attached directly to framing		PROPERTY ADDRESS		
16d (16 penny)	0.162" x 3-1/2" long	R.		
20d (20 penny)	0.192" x 4" long			
Zod (Zo pomij)	O. TOZ. X. T. TOTIG			
Structural wood	Simpson Strong-Tie 1/4" SDS, GRK			
screws	3/8" RSS "Climatek", USP Mitek 1/4"	<u>s</u>		
	WS "Gold Coat", or equivalent.	a.	et	
3" screw	3" long structural wood screw	et	ં જે 🖰	
4" screw	4" long structural wood screw		J ⊑ J	
6" screw	6" long structural wood screw	n	= a	
		∺ <u>;</u>	⊕ ≅	
Wood structural	15/32" Plywood sheathing. Exposure)	o 20 o	
panel	rating of Exterior or Exposure I.	<u>ב</u>	t a G	
	4- or 5-Ply.	٥	≟ = ≣	_
	Or Oriented Strand Board (OSB)	Ö	(a)	ĺά
	Oriented Strand Board (OSB) Conforming to US Voluntary Product	=		7
	Standard PS 2. Exposure rating of	8		삙
	Exterior or Exposure I. Manufactured	>	a	5
	with exterior glue. Minimum 4-ply.	<u>e</u>	없 등 등	기
		dd	Crawlspace Dwellings -Based Seismic Assessment an One- and Two-Family Dwellings	Issued: JUNE 2021
LVL (laminated	Weyerhauser "Microllam",	ΪΞ	Հ Ջ -	ijĮ
veneer lumber)	Boise-Cascade "VersaLam",	0	a g ≤	<u>88</u>
	Georgia-Pacific "GP-Lam",	Q	- 38 е В 88 е	
	LP "Solid Start", or equivalent.	D	ပ မှု ည	
	A	Floor Framing to Cripple Wall Connection Details	fit of Crawlspace Dwellings (Plan Set rability-Based Seismic Assessment and Retrofit of One- and Two-Family Dwellings	
Plate washer	3" x 3" square x 0.229" thick.	Ē	≒	
		<u> </u>	T = =	
"Peel & Stick"	Fortiflash, Orange Peel-n-Seal,	ш.	Retro i	
flashing tape	Typar, Tyvek, Vycor, HardieWrap, or	ō	\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\overline{\ov	
bituthene	equivalent.	<u> </u>	🗠	
		ш —		
				\neg
AB	BREVIATIONS		/	_

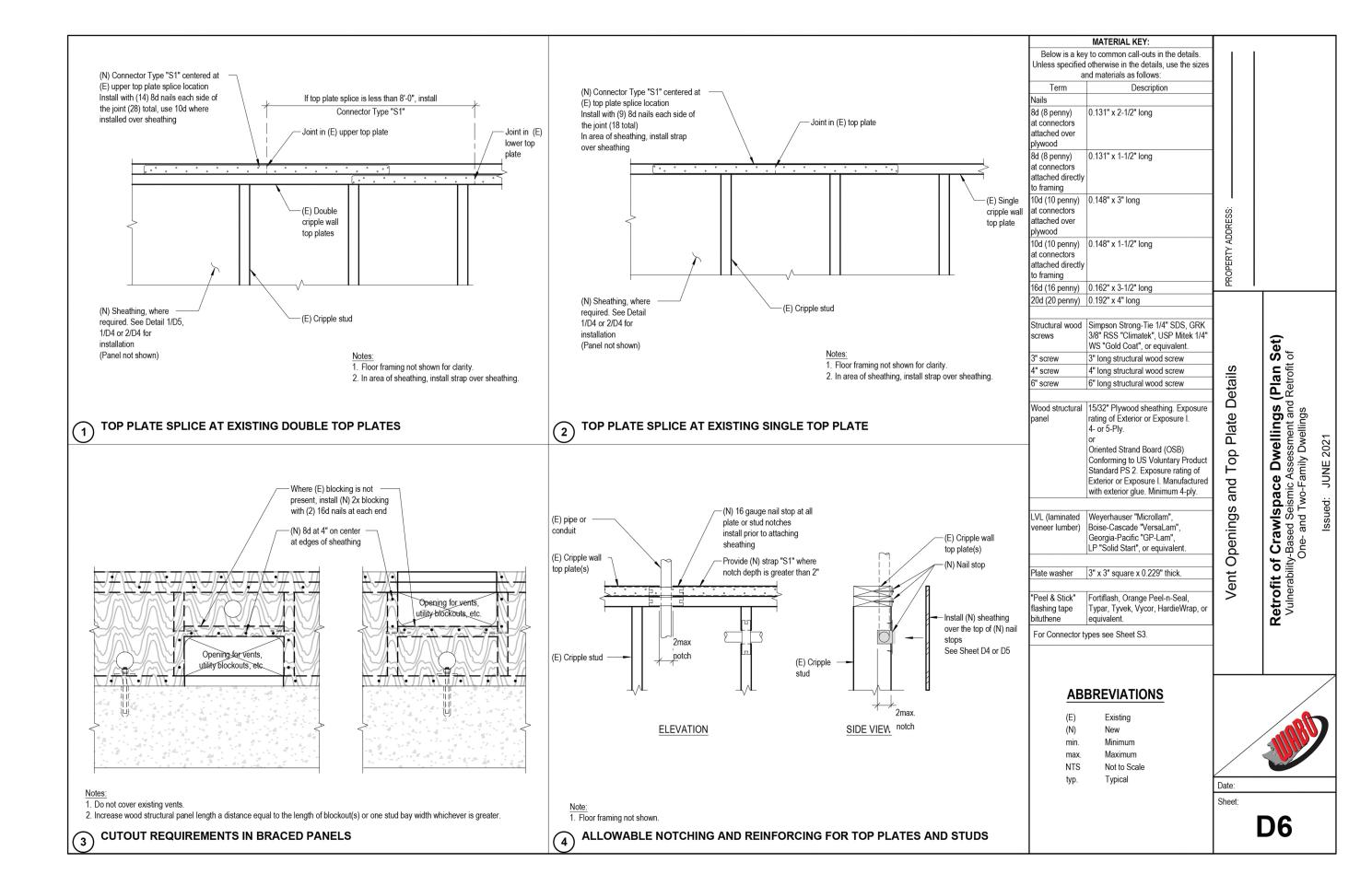
(E) Existing (N) New Minimum min. Maximum max. NTS Not to Scale typ. Typical

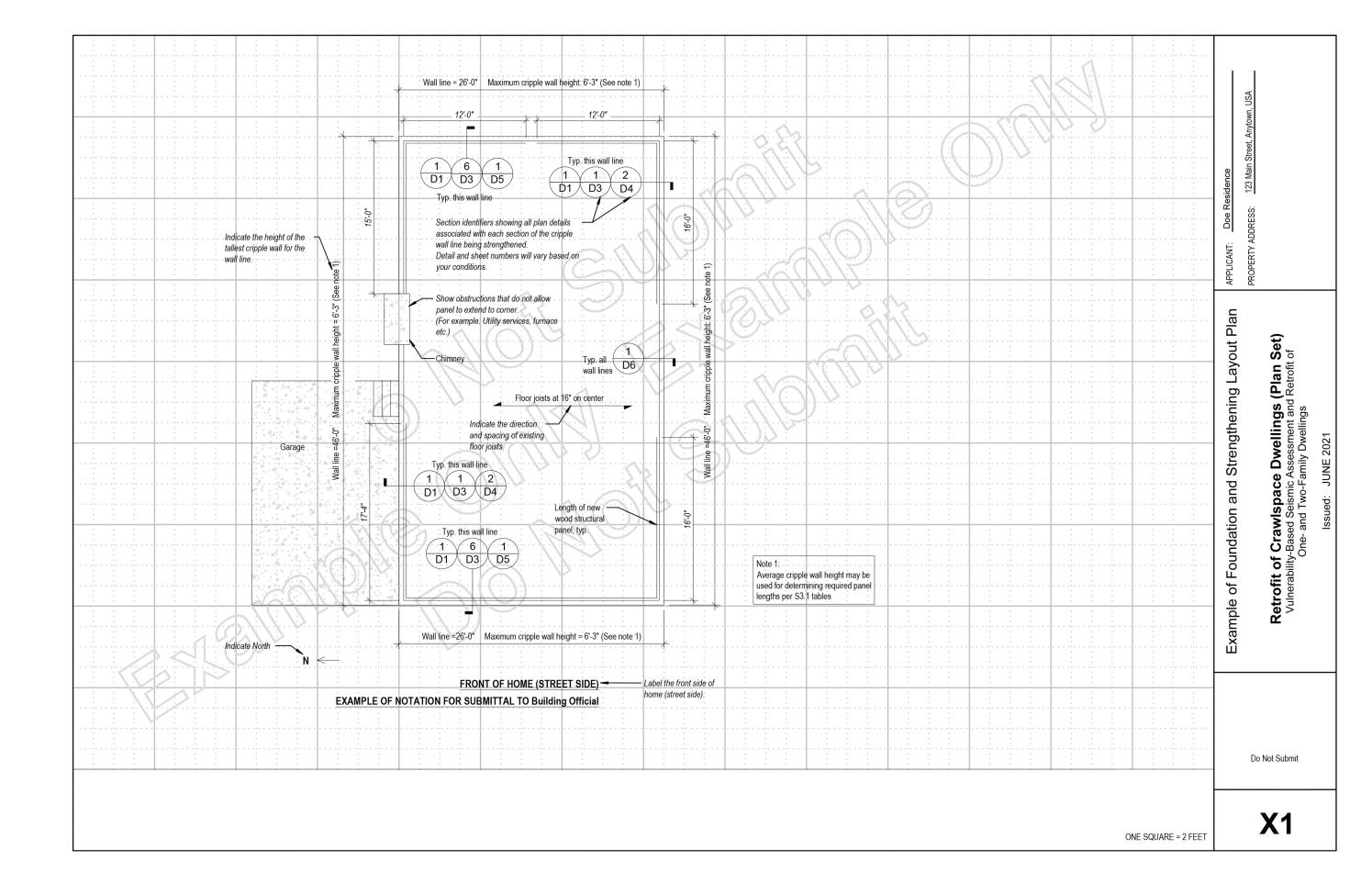


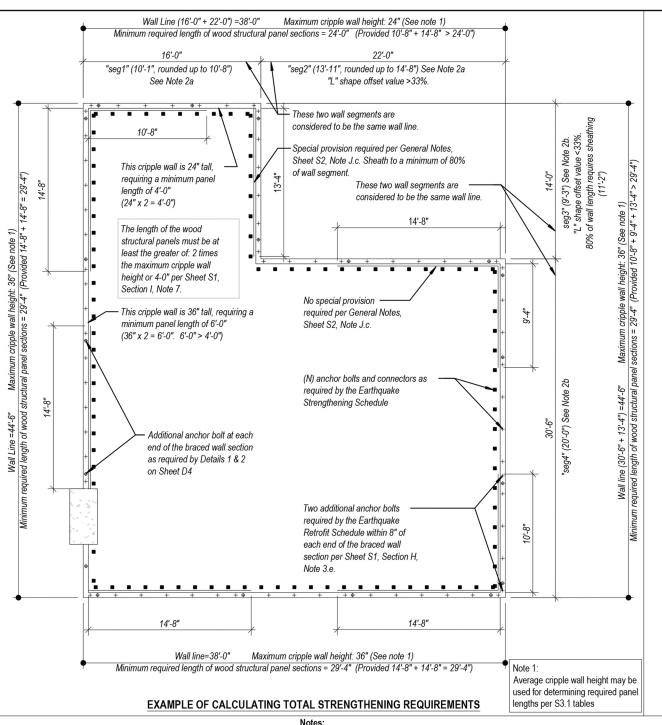
D3.2

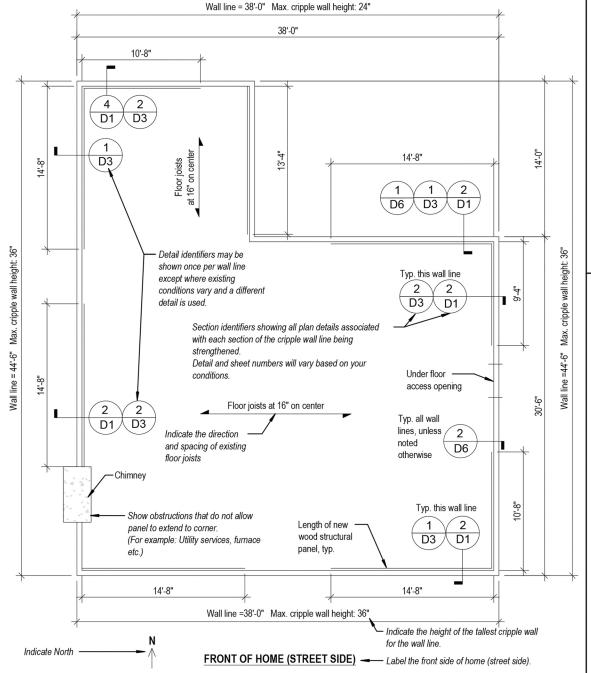












- 1. Plans show a typical building foundation. Instructional notes are in *italics* and should not be included on submittal drawing. See Sheet X1 for an example of a final plan. Refer to Sheet 01 for additional instructions.
- 2. Sample calculations for offset walls as shown above (See Section J of the General Notes on Sheet S1):
- 2a. Approximate length of strengthening required for segments "seg1" and "seg2" at the north wall line: $seg1 = 24'-0" \left(\frac{16'}{16'+22'}\right) \approx 10'-1" \qquad seg2 = 24'-0" \left(\frac{22'}{16'+22'}\right) \approx 13'-11"$
- 2b. Length of strengthening required for segments "seg3" and "seg4" at the east wall line: $seg3 = 29'-4" \left(\frac{14'}{30'-6"+14'} \right) \approx 9'-3" \quad seg4 = 29'-4" \left(\frac{30'-6"}{30'-6"+14'} \right) \approx 20'-1"$
- 3. Required length values are rounded up to be at 16" typical stud space increments

- Additional foundation sill anchor or connector

-Detail Number (Homeowner or Contractor to -Sheet Number choose most appropriate detail)

EXAMPLE OF NOTATION FOR SUBMITTAL TO BUILDING OFFICIAL

Definitions:

Existing

Strengthening: Foundation and/or cripple wall work intended to improve performance during an earthquake.

dimension on one side.

Official.

Building (

submit to the

not

ဓ

This sheet is for instruction and reference only.

weight classification in a very high seismic area. Using the S DS 1.5/One-Story Earthquake Strengthing Schedule from

This example is a 1383 square foot, 1-story home with a "Light"

Sheet S3.1, the 1201-1500 Total Area row shows that the length of strengthening required is 29'-4" (14.7x2) at the three 36" high cripple wall lines and 24'-0" (12x2) at the 24" high cripple wall line. The wall lines of this home allow enough length to use wood structural panels

This example uses 1/2"ø Anchor Bolts (17 required per wall line), and Type "E" Connectors (25 required per wall line).

Key:

Minimum required length of strengthening using wood structural panels, anchors, and connectors

- + Foundation sill anchor or connector
- Floor connector

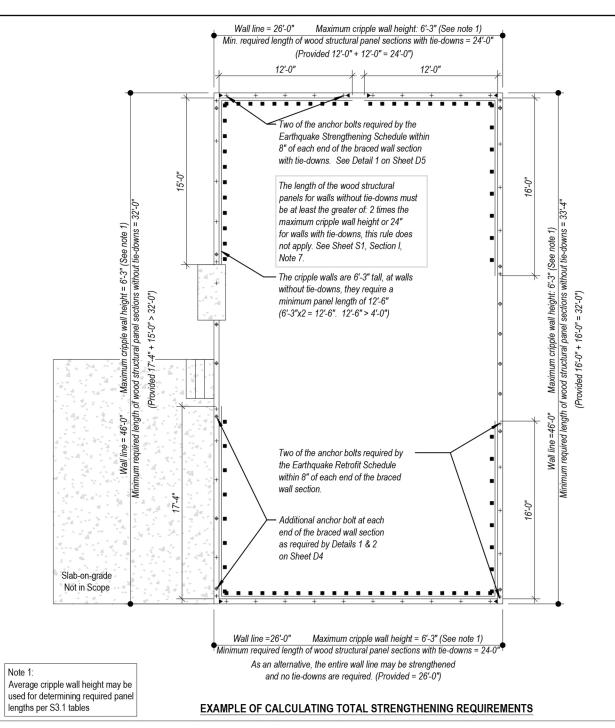
(E)

All wall segments forming the overall building

Example - Foundation Plan (Dwelling without tie-downs)
Consult a Registered Design Professional where plan offset is
greater than 33% of the pargest plan dimension
Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of

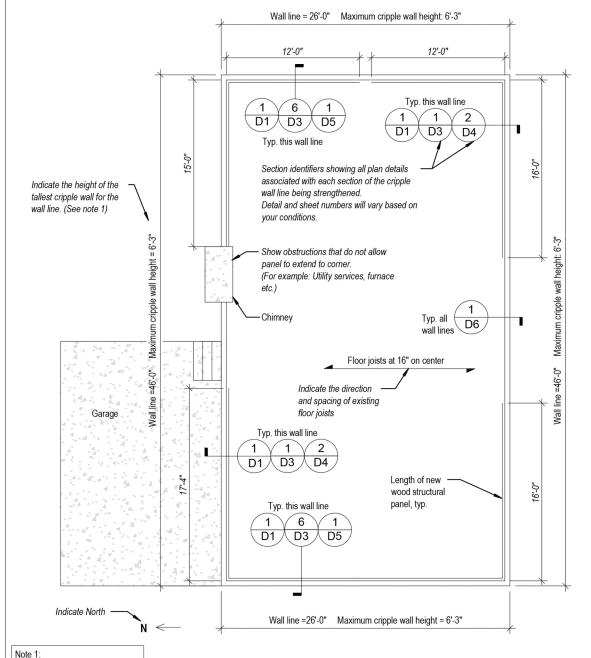
ed Seismic Assessment and and Two-Family Dwellings

JUNE 2021



This example is a 2398 square foot two-story home (excluding garage and porch), with a "Light" weight classification in a low seismic area. Using the SDS 1.0/Two-story Earthquake Strengthening Schedule from Sheet S3.1, the 2001-2400 Total Area row shows that the length of strengthening required is 32'-0" (16x2) for the uniform cripple wall height of 6'-3". The side walls allow enough length to use wood structural panels without tie-downs. The front and back walls do not provide enough length and will require tie-downs, this will reduce the required length to 24'-0" (12x2). This example uses 5/8"ø Anchor Bolts (10 required per wall line) and Type "D" Connectors (23 required per wall line).

- 1. Plan shows typical notation. Instructional notes are in *italics* and should not be included on submittal drawing. Refer to Sheet 01 for additional instructions.
- 2. Required length values are rounded up to be at 16" typical stud space increments.



Key:

Average cripple wall height may be

used for determining required panel

lengths per S3.1 tables

Minimum required length of strengthening using wood structural panels, anchors, and connectors

- + Foundation sill anchor bolt or connector
- Additional foundation sill anchor or connector
- ► Tie-down
- Floor framing connector

·Detail Number -Sheet Number

(Homeowner or Contractor to choose most appropriate detail)

EXAMPLE OF NOTATION FOR SUBMITTAL TO Building Official

Definitions:

FRONT OF HOME (STREET SIDE)

(E) Existing

Strengthening: Foundation and/or cripple wall work intended to

improve performance during an earthquake.

Label the front side of

home (street side).

Wall line: All wall segments forming the overall building

dimension on one side.



Example - Foundation Plan (Dwelling with tie-downs)

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

Official.

not submit to the Building

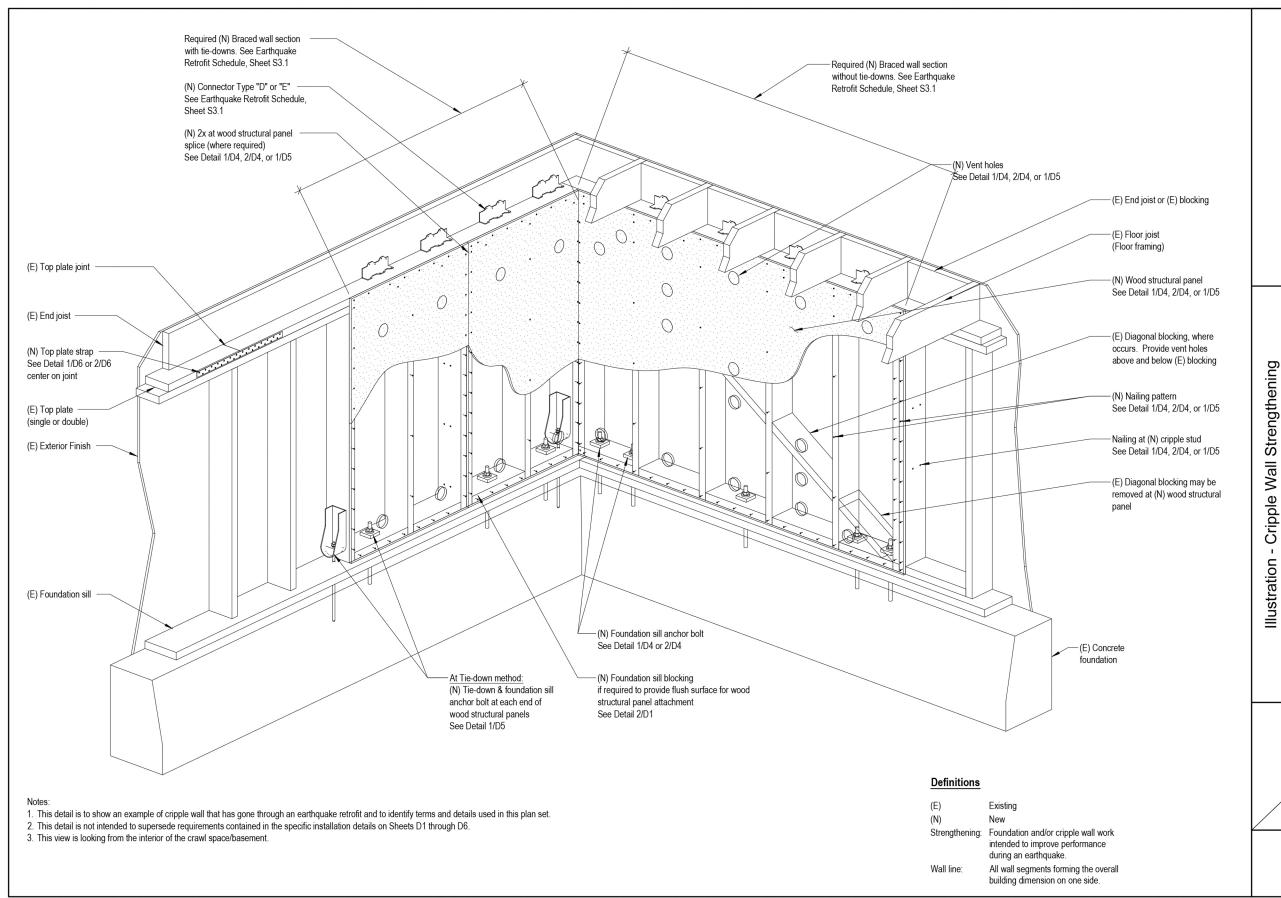
മ

instruction and

only.

s sheet is for ins reference c

This



This sheet is for instruction and reference only.

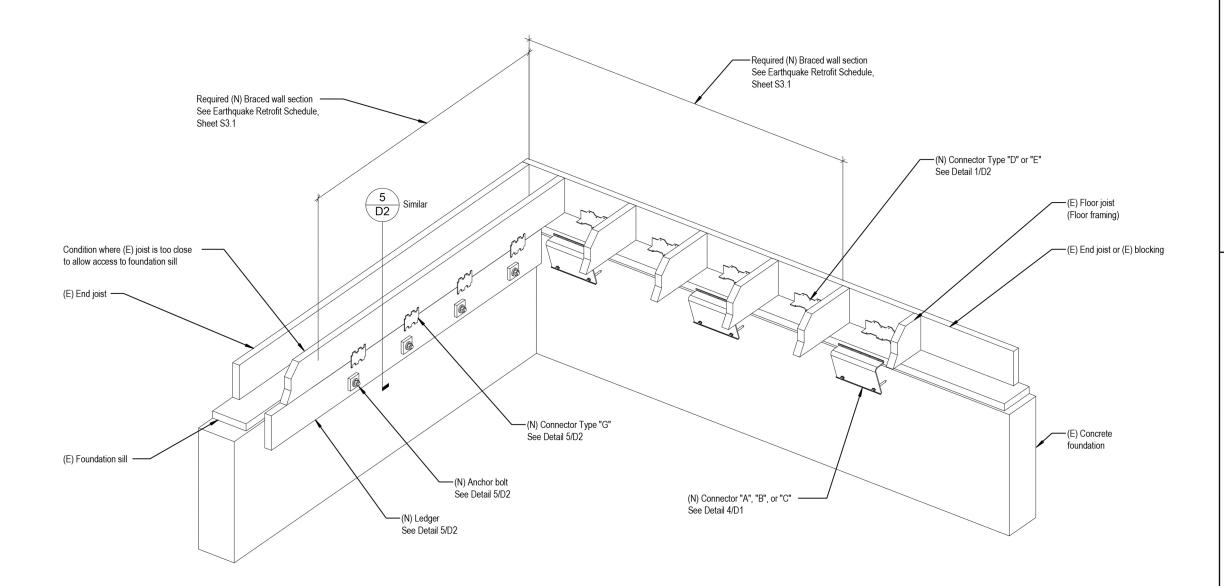
Official.

Building (

not submit to the

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

X4



- (N)

Strengthening: Foundation and/or cripple wall work

All wall segments forming the overall Wall line: building dimension on one side.

X5

Do not submit to the Building Official.

Retrofit of Crawlspace Dwellings (Plan Set)
Vulnerability-Based Seismic Assessment and Retrofit of
One- and Two-Family Dwellings

JUNE 2021

Illustration - Strengthening - No Cripple Wall

This sheet is for instruction and reference only.

Definitions

(E) Existing

intended to improve performance

during an earthquake.

Notes:

1. This detail is to show an example of an earthquake retrofit where there is no cripple wall, and to identify terms and details used in this plan set.

2. This detail is not intended to supersede requirements contained in the specific installation details on Sheets D1 through D6.

3. This view is looking from the interior of the crawl space.